APPENDIX F Safe Additional Available Drawdown

APPENDIX F

SAFE ADDITIONAL AVAILABLE DRAWDOWN

Safe additional available drawdown is defined as the additional depth water within a pumping well could fall and still maintain that well's Allocated pumping rate. It is calculated as the additional drawdown that is available beyond the drawdown created by the Existing conditions pumping rate. To establish the safe additional available drawdown for each municipal well within the City of Guelph and the Township of Guelph/Eramosa (Rockwood and Hamilton Drive), the following components need to be evaluated or calculated for each well:

- Safe water level elevations: the lowermost elevation to which an operator can pump the water levels in municipal pumping well. This elevation may be related to the well screen elevation, pump intake elevation, depth of water bearing zones, or similar operational limitations.
- Existing water level elevations in the pumping wells: the elevation of the observed average annual
 pumped water level within each municipal well for 2008 (for Guelph) and a representative low
 operating water level determined for municipal wells in Rockwood and Hamilton Drive
 (Burnside 2015a) during periods of normal operation.
- Estimated non-linear well losses at each well: drawdown within the well in response to well inefficiencies (e.g., entrance losses or turbulent flow around pump fittings) created during groundwater extraction.

1 SAFE WATER LEVEL ELEVATION

The safe water level elevation at each municipal water supply well was developed with City of Guelph and the Township of Guelph/Eramosa staff and their consultants. The safe water levels or set points for operation are based on historical and recent testing and operation of the wells and have largely been determined based on pump intake levels or the top of the open borehole in bedrock wells. For wells where there is a lack of current operational data, set points were estimated from the depths of major producing fracture zones or aquifers. Safe water level elevations are summarized in Table F-1.

2 EXISTING WATER LEVEL ELEVATIONS

The average pumped water level elevation in each City of Guelph municipal well in 2008 was determined by examining water level hydrographs of each municipal pumping well. Operating low water levels were determined by consultants of the Township of Guelph/Eramosa for the existing Rockwood and Hamilton Drive municipal wells using automatic water level recorders (Burnside 2015a). Where necessary, professional judgement was used to determine a level that was representative of pumped conditions at a typical pumping rate where adjacent wells were also pumping at typical rates. Figures F1 to F24 illustrate the derivation of the existing water level elevations.

The safe additional available drawdown is a measure of the additional drawdown within a well, regardless of the non-linear head losses at each well due to turbulent flow of water through the well screen and casing to the pump intake. The safe additional available drawdown is calculated as the difference between the average pumped water level and the safe water level. Safe additional available

drawdown values are summarized in Table F-1. Figures F1 to F24 summarize the derivation of safe additional available drawdown in the Guelph, Rockwood, and Hamilton Drive wells. No transient water level and pumping data was available for Arkell Well 14, Arkell Well 15, Sacco Well, and Smallfield Well; therefore, no summary figures are provided for these wells.

TABLE F-1 Safe Additional Available Drawdown

Well	Safe Water Level (m asl)	Average Pumped Water Level (m asl)	Safe Additional Available Drawdown (m) ⁶					
City of Guelph								
Arkell 1	320.7 ¹	322.9 ³	2.2					
Arkell 14	304.7 ¹	316.0 ³	11.3					
Arkell 15	306.3 ¹	317.6 ³	11.3					
Arkell 6	298.5 ¹	313.7 ³	15.2					
Arkell 7	301.0 ¹	313.9 ³	12.9					
Arkell 8	303.4 ¹	313.9 ³	10.5					
Burke	313.1	319.2 ³	6.1					
Calico	291.8 ¹	309.1 ³	17.3					
Carter Wells	317.6 ¹	320.2 ³	2.6					
Clythe Creek	294.5 ¹	321.4 ³	26.9					
Dean Ave.	277.2 ¹	289.7 ³	12.5					
Downey Rd.	282.4 ¹	297.0 ³	14.6					
Emma	288.7 ¹	293.2 ³	4.5					
Helmar	295.5 ¹	303.5 ³	8.0					
Membro	274.4 ¹	287.7 ³	13.3					
Paisley	282.6 ¹	299.1 ³	16.5					
Park 1 and 2	283.1 ¹	291.8 ³	8.7					
Queensdale	278.3 ¹	289.9 ³	11.6					
Sacco	307.8 ¹	337.9 ³	30.1					
Smallfield	287.2 ¹	334.2 ³	47.0					
University	278.0 ¹	293.0 ³	15.0					
Water Street	276.8 ¹	290.4 ³	13.6					
Rockwood								
Rockwood Well 1	329.3 ²	344.0 ⁴	14.7					
Rockwood Well 2	330.6 ²	345.0 ⁴	14.4					
Rockwood Well 3	317.7 ⁷	331.0 ⁴	13.3					
Rockwood Well 4	321.0 ²	355.1 ⁵	34.1					
Hamilton Drive								
Cross Creek	303.7 ²	317.0 ⁴	13.3					
Huntington	303.6 ²	314.0 ⁴	10.4					

Notes:

¹ Defined by the City of Guelph.

² Selected as 1 m above pump intake (Burnside 2015a).

³ Observed from water levels measured during periods of normal well operation and production (2008 - Guelph)

⁴ Selected using operating low water levels from automatic water level recorder data (Burnside 2015a)

⁵ Rockwood Well 4 was commissioned in 2016 and was not pumped during Existing conditions. Therefore, the water level used to determine the safe additional drawdown is the static water level determined during well completion (January 2015; Burnside 2015b).

⁶ Relative to average observed water level

⁷ Selected as 1 m above the bottom of the casing (top of open bedrock interval)

3 NON-LINEAR IN-WELL LOSSES

Well losses refer to the difference between the theoretical drawdown in a well and the observed drawdown and are due to factors such as turbulence in the well itself as water flows into the pump. These well losses need to be considered in the Tier Three Assessment as the safe additional available drawdown refers specifically to the water level in the well and not the average water level in the aquifer near the well. The in-well losses are calculated as the additional drawdown that is expected within the pumping well due to the incremental increase from the Existing to the Allocated pumping rates.

The two components of observed additional drawdown in a given pumping well are described in **Equation 1** (Bierschenk 1963; Hantush 1964; Jacob 1947):

$$s = BQ + CQ^2$$
 Equation 1

Where s is drawdown, Q is the pumping rate, B is the aquifer loss coefficient (Theis 1935), and C is the well loss coefficient, which is constant for a given pumping rate. The first term in the equation (BQ) describes the linear component of the drawdown (i.e., doubling the pumping rate leads to a doubling of the drawdown). This term accounts for the drawdown in the formation near the well. The second term of the equation (CQ^2) describes the non-linear well-loss component of drawdown (Jacob 1947) in the well itself; this is the additional component that was quantified in this assessment.

Well losses are estimated using step test results. Step tests are hydraulic tests where a well is pumped at a series of different pumping rates and the drawdown throughout the test is recorded. Non-linear well loss coefficients were estimated using the step test results presented in Stantec Consulting Ltd.'s (2009) Sacco and Smallfield Report, Jagger Hims Limited (1995, 1998a, 1998b, 1998c; i.e., the "Quadrant Reports"), Gartner Lee Limited (2003), Lotowater Geoscience Consultants Inc. (2005), R.J. Burnside and Associates Limited (2002, 2015b) and Well Initiatives Limited (2012). Where no step test data was available for the municipal wells, or where data was considered unreliable, a non-linear well loss coefficient of 3.82E-07 m/(m³/day)² was selected based on the assumption that the well is mildly deteriorated (Walton 1962).

The loss coefficient, *C*, is calculated directly from step test data following the technique developed by Kasenow (1998):

$$C = \frac{s_2 Q_1 - s_1 Q_2}{Q_1 Q_2^2 - Q_2 Q_1^2}$$
.....Equation 2

Where:

 s_1 is the total stabilized drawdown at the end of pumping step 1 Q_1 is the pumping rate for step 1

s₂ is the total stabilized drawdown at the end of pumping step 2

Q₂ is the pumping rate for step 2

For each step test, these coefficients were calculated for consecutive steps and then averaged to determine the loss coefficient for the well at the time of the step test. **Equation 2** after Jacob (1947) was

used to calculate drawdown due to in-well head losses for the increased pumping from Existing to the Allocated rates:

$$\Delta S_{inwell} = {\it C} \; [(Q_{EC} + \Delta Q)^2 \; - \; Q_{EC}^2] \;$$
 Equation 3

Where: C is the well loss coefficient determined from step-test data,

Q_{FC} is the Existing Conditions (2008 - Guelph; 2009 to 2010 - Rockwood and Hamilton Drive)

pumping rate used in the base case steady-state model,

 ΔQ equal to the increase from Existing Conditions (2008 - Guelph; 2009 to 2010 - Rockwood and

Hamilton Drive) pumping rate to the Allocated municipal pumping rate used in the Risk

Assessment scenarios.

Drawdown from in-well head losses values were derived for two sets of Allocated municipal pumping rates for the City of Guelph wells:

- The Existing plus Committed Allocated municipal pumping rates used in the steady-state Risk Assessment scenarios G(1) and G(2), and as the minimum Allocated rates for the transient Risk Assessment scenarios H(1) and H(2).
- The maximum Allocated pumping rates used in the transient Risk Assessment scenarios H(1) and H(2) when the Glen Collector was not supplying its Allocated rate.

The amount of drawdown from in-well head losses was derived for a single set of Allocated pumping rates for Rockwood and Hamilton Drive municipal pumping wells. Minimum and maximum Allocated rates were not needed for these wells as their pumping is not tied to the demand at the Glen Collector.

Pumping rates used for the in-well head loss calculations are shown in Table F-2. Non-Linear head losses are summarized in Table F-3.

TABLE F-2 Pumping Rates for Non-linear Head Loss Calculations

Well	Typical Pumped Rates (2008)	Existing Plus Committed	ΔQ between Existing Conditions & Existing plus Committed	Maximum Allocated Rates for Long-Term Drought Scenarios	ΔQ between Existing Conditions & Maximum Allocated Rates		
	m³/day	m³/day	m³/day	m³/day	m³/day		
	City of Guelph						
Arkell 1	730	1,400	670	1,400	670		
Arkell 14	-	3,300	3,300	4,400	4,400		
Arkell 15	-	3,300	3,300	4,400	4,400		
Arkell 6	3,774	4,900	1,126	5,300	1,526		
Arkell 7	3,689	4,900	1,211	5,300	1,611		
Arkell 8	3,694	4,900	1,206	4,900	1,206		
Burke	5,385	6,000	615	6,300	915		
Calico	748	1,100	352	1,100	352		
Carter Wells	3,400 ¹	4,000	600	4,400	1,000		
Clythe Creek	-	2,200	2,200	2,200	2,200		
Dean Ave.	1,215	1,500	285	1,500	285		
Downey Rd.	3,940	5,100	1,160	5,236	1,296		
Emma	2,600 ¹	2,100	-500	2,400	-200		
Helmar	800 ¹	1,100	300	1,200	400		
Membro	3,036	4,200	1,164	4,300	1,264		
Paisley	762	800	38	1,000	238		
Park 1 & 2	6,400 ¹	6,400	-	6,900	500		
Queensdale	702	2,000	1,298	2,000	1,298		
Sacco	-	1,150	1,150	1,200	1,200		
Smallfield	-	1,400	1,400	1,400	1,400		
University	1,648	2,500	852	2,500	852		
Water Street	1,184	2,300	1,116	2,400	1,216		
Rockwood							
Rockwood Well 1	283 ²	396	112	n/a	n/a		
Rockwood Well 2	262 ²	367	105	n/a	n/a		
Rockwood Well 3	422 ²	572	150	n/a	n/a		
Rockwood Well 4	0 ³	572	572	n/a	n/a		
Hamilton Drive							
Cross Creek	87 ²	90	3	n/a	n/a		
Huntington	92 ²	95	3	n/a	n/a		

¹ Rates used in the Risk Assessment Scenarios are based on the typical pumped rates for which a typical pumped water level has been observed, rather than the 2008 average pump rate used for the water demand calculations.

² Rates used in the Risk Assessment Scenarios are based on the 2009 to 2010 average pump rate.

³ Rockwood Well 4 was commissioned in 2016 and was not pumped during the Study Period (2009 to 2010)

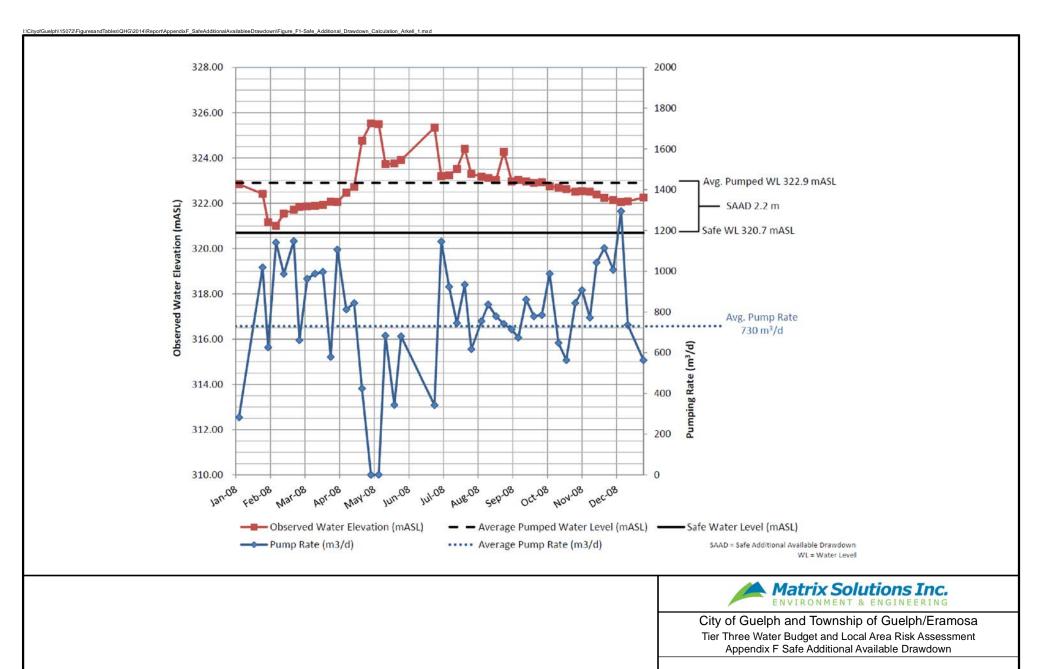
TABLE F-3 Estimated Drawdown due to Non-linear Head Losses

	Well Loss	Existing Conditions versus Existing Plus Committed		Existing Conditions versus Maximum Allocated Rates for Long-Term Drought Scenarios	
Well	Coefficient (C)	Pumping Rate Increase (ΔQ)	Drawdown due to Non-Linear Head Losses	Pumping Rate Increase (ΔQ)	Drawdown due to Non-Linear Head Losses
	m/(m³/day)²	m³/day	m	m³/day	m
City of Guelph					
Arkell 1	2.38E-07	670	0.3	670	0.3
Arkell 14	1.88E-08	3,300	0.2	4,400	0.4
Arkell 15	1.12E-08	3,300	0.1	4,400	0.2
Arkell 6	5.83E-08	1,126	0.6	1,526	0.8
Arkell 7	1.66E-08	1,211	0.2	1,611	0.2
Arkell 8	6.87E-09	1,206	0.1	1,206	0.1
Burke	1.71E-07	615	1.2	915	1.8
Calico	2.91E-08	352	0.0	352	0.0
Carter Wells	1.74E-07	600	0.2	1,000	0.3
Clythe Creek	2.72E-06	2,200	13.2	2,200	13.2
Dean Ave.	3.28E-06	285	2.5	285	2.5
Downey Rd.	8.51E-08	1,160	0.9	1,296	1.0
Emma	1.92E-07	-500	-0.5	-200	-0.2
Helmar	1.36E-07	300	0.1	400	0.1
Membro	1.64E-07	1,164	1.4	1,264	1.5
Paisley	2.98E-06	38	0.2	238	1.3
Park 1 and 2	3.40E-08	-	0.0	500	0.2
Queensdale	5.58E-08	1,298	0.2	1,298	0.2
Sacco	4.59E-07	1,150	0.6	1,200	0.7
Smallfield	3.60E-06	1,400	7.1	1,400	7.1
University	4.46E-07	852	1.6	852	1.6
Water Street	9.91E-07	1,116	3.9	1,216	4.3
Rockwood					
Rockwood Well 1	3.82E-07	112	0.0	n/a	n/a
Rockwood Well 2	3.82E-07	105	0.0	n/a	n/a
Rockwood Well 3	3.22E-06	150	0.5	n/a	n/a
Rockwood Well 4	9.20E-06	572	1.4	n/a	n/a
Hamilton Drive					
Cross Creek	3.82E-07	3	0.0	n/a	n/a
Huntington	8.38E-06	3	0.0	n/a	n/a

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Safe Additional Drawdown Calculation Arkell 1

To Jane:

17 Jan 2017

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J. Melchin

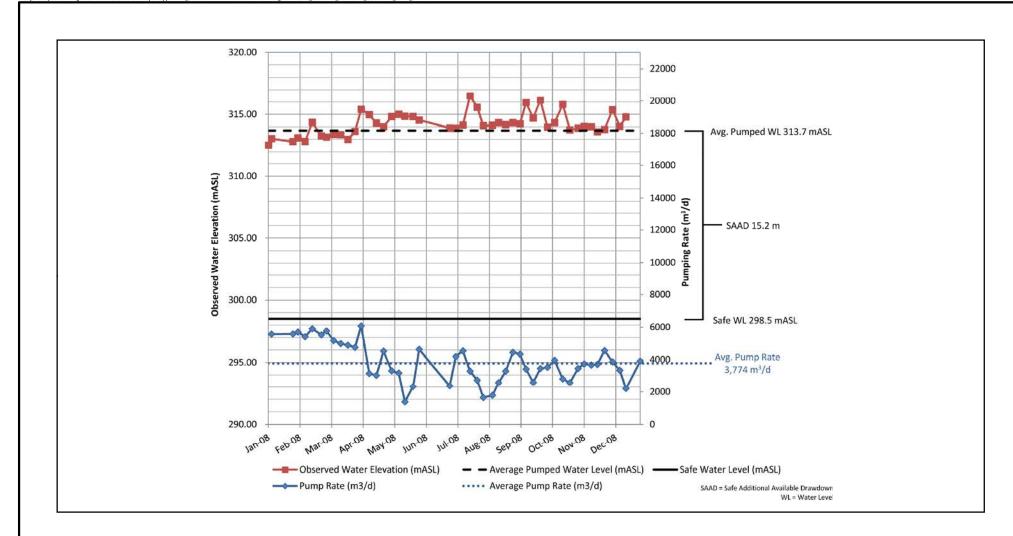
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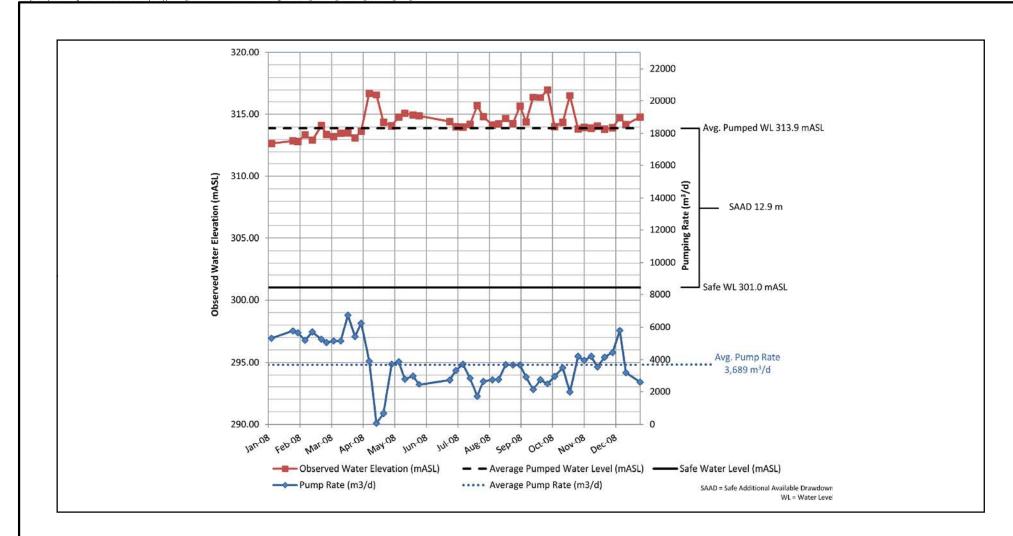
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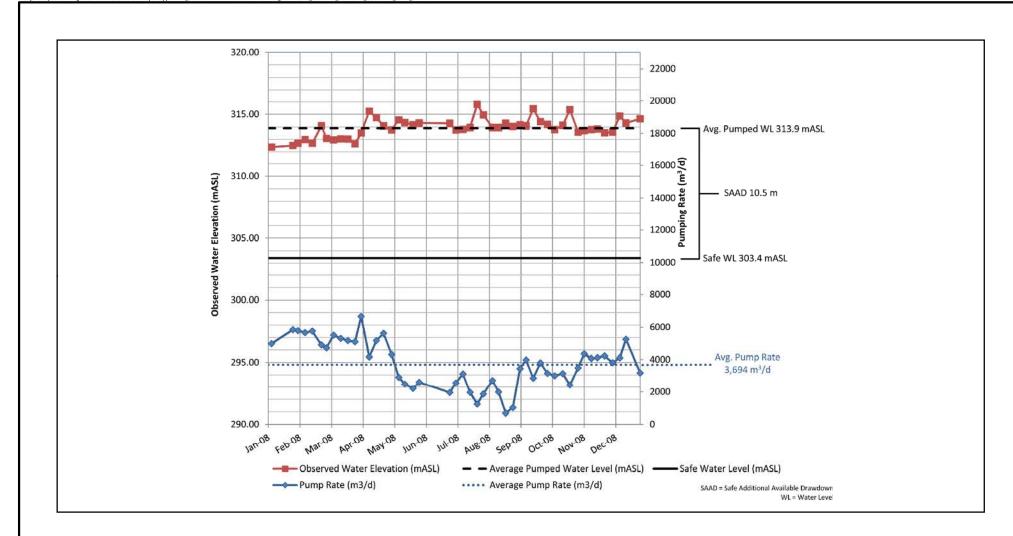
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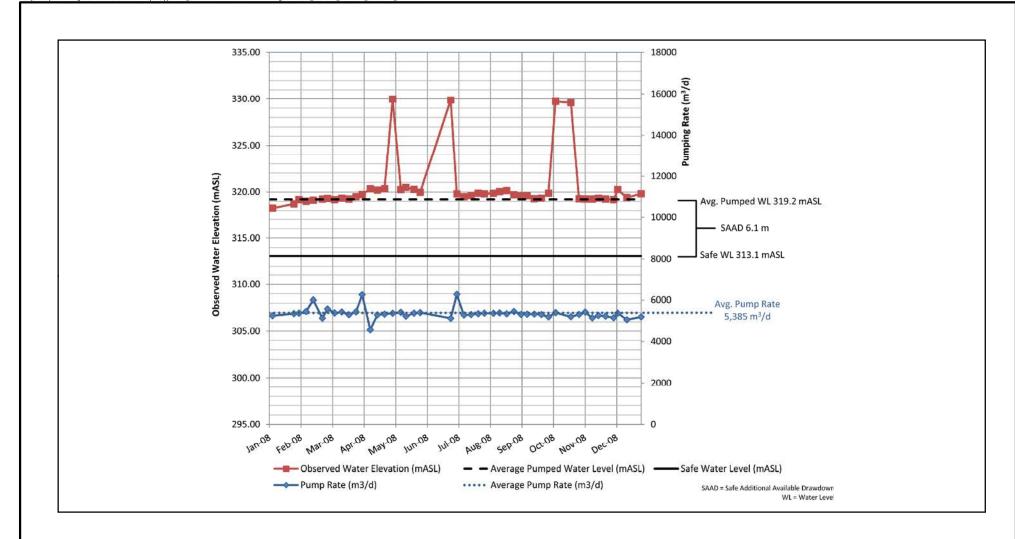
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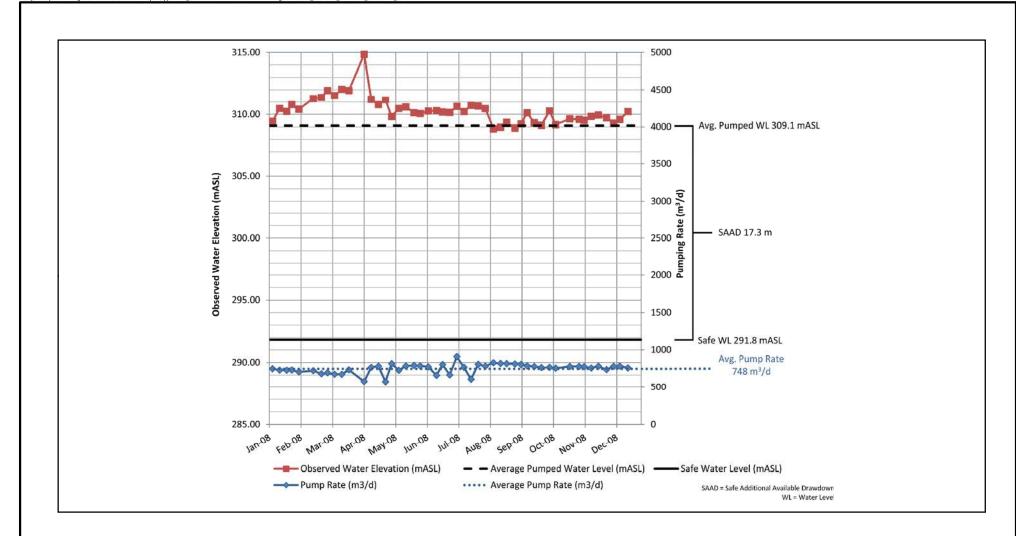
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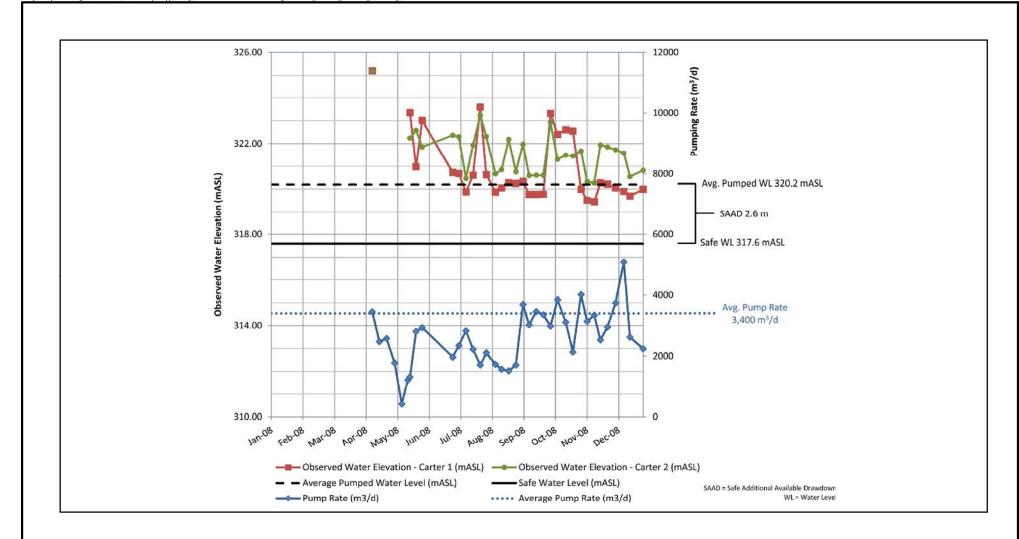
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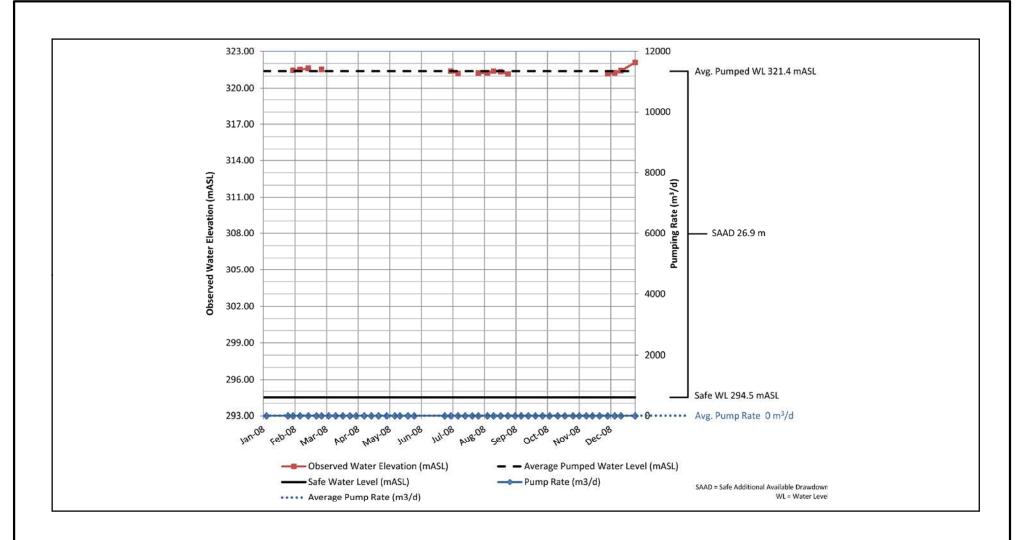
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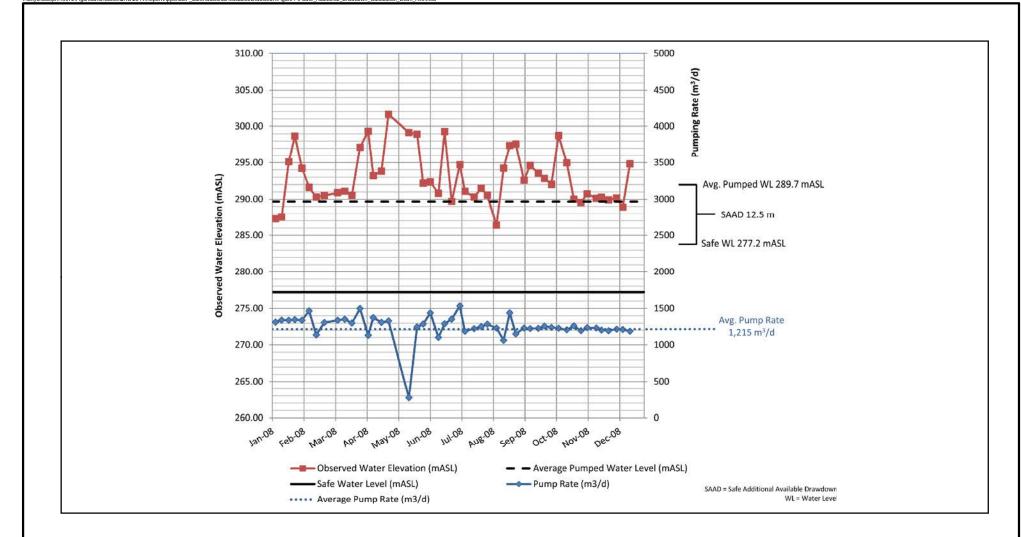
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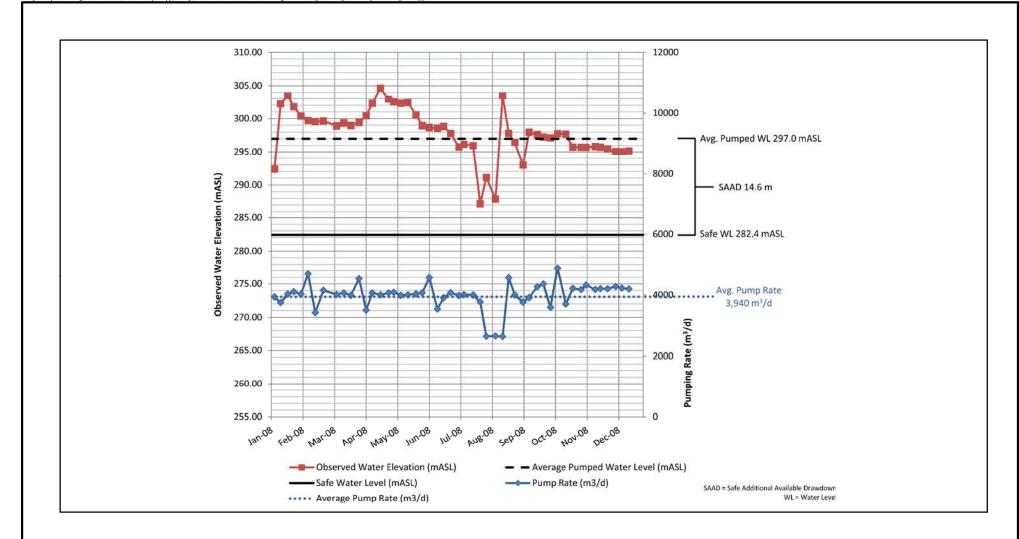
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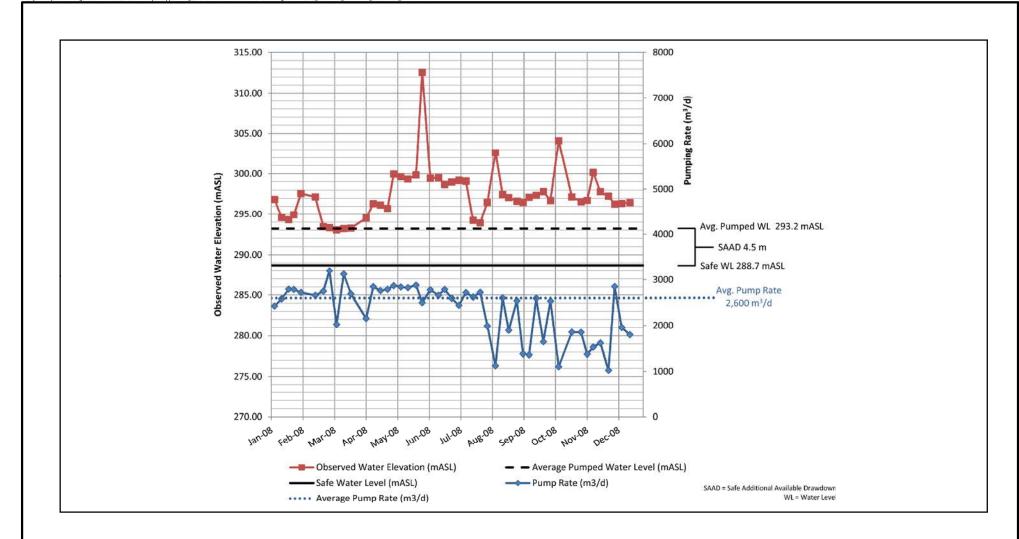




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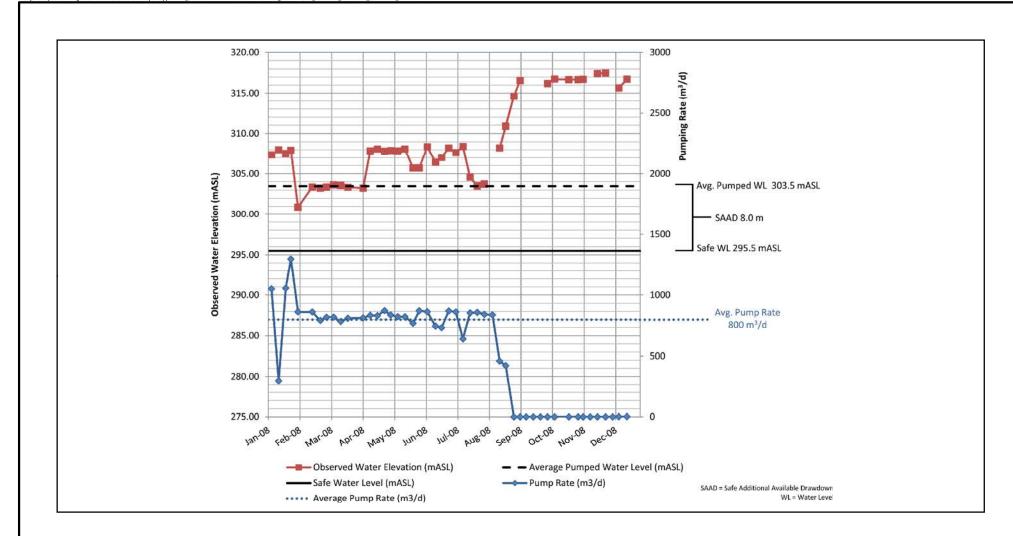
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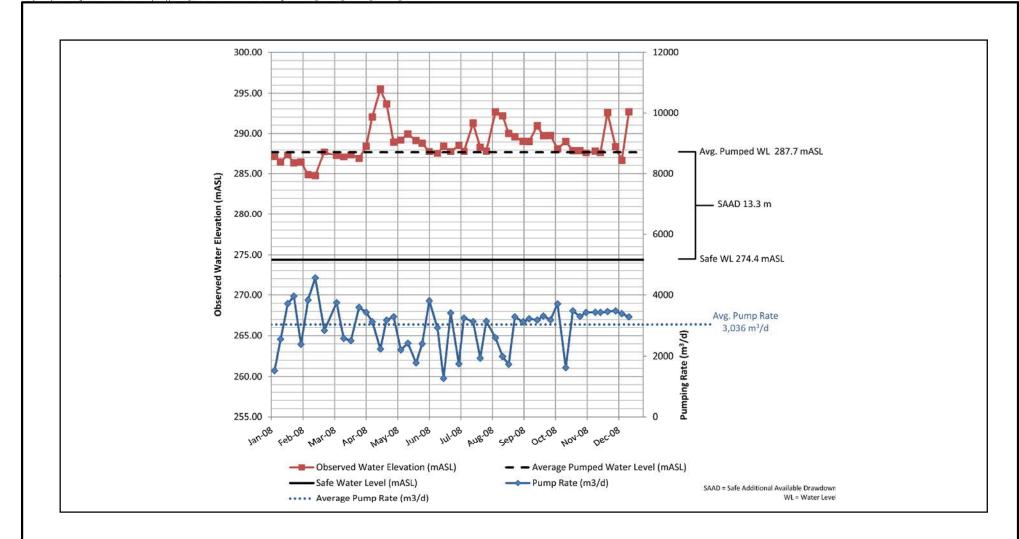
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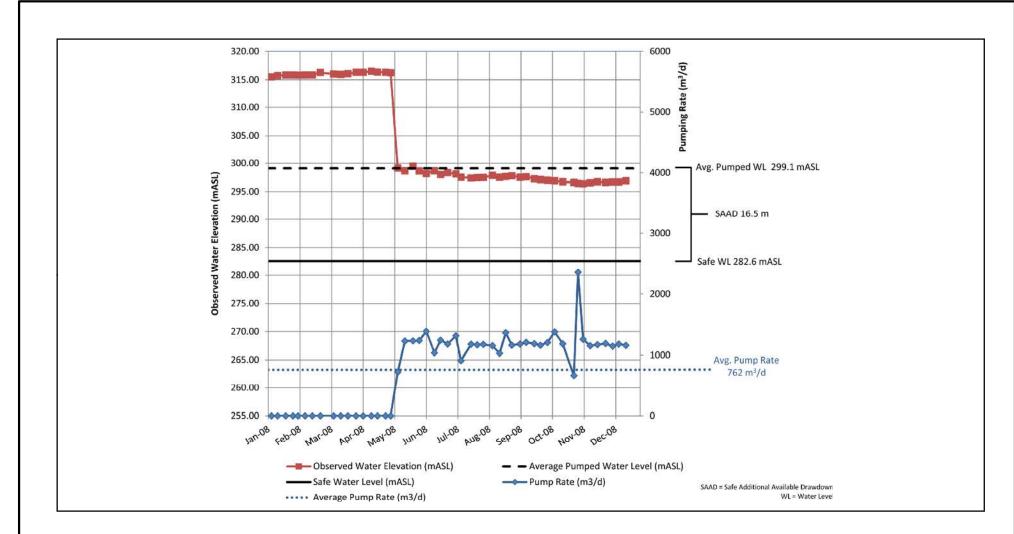
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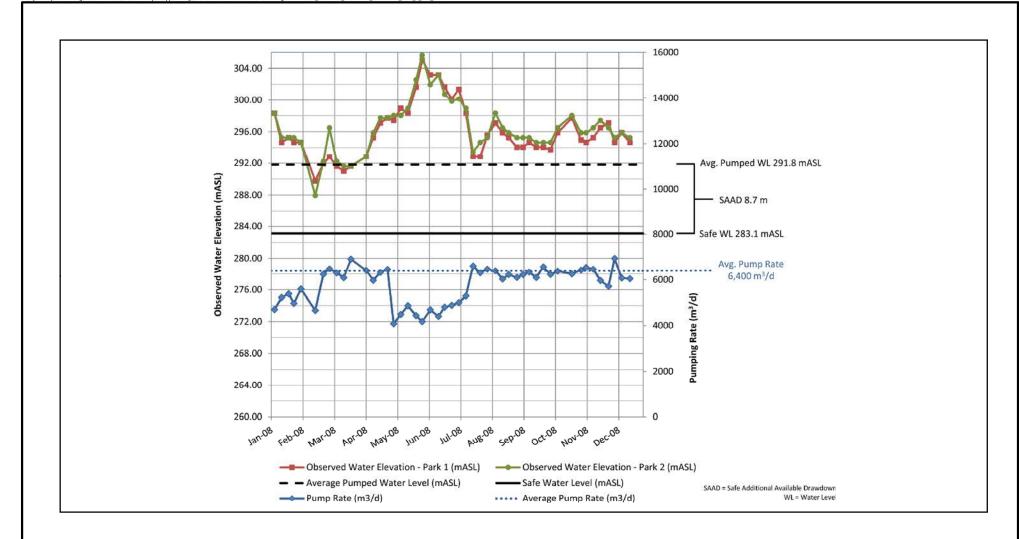
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Safe Additional Drawdown Calculation Paisley

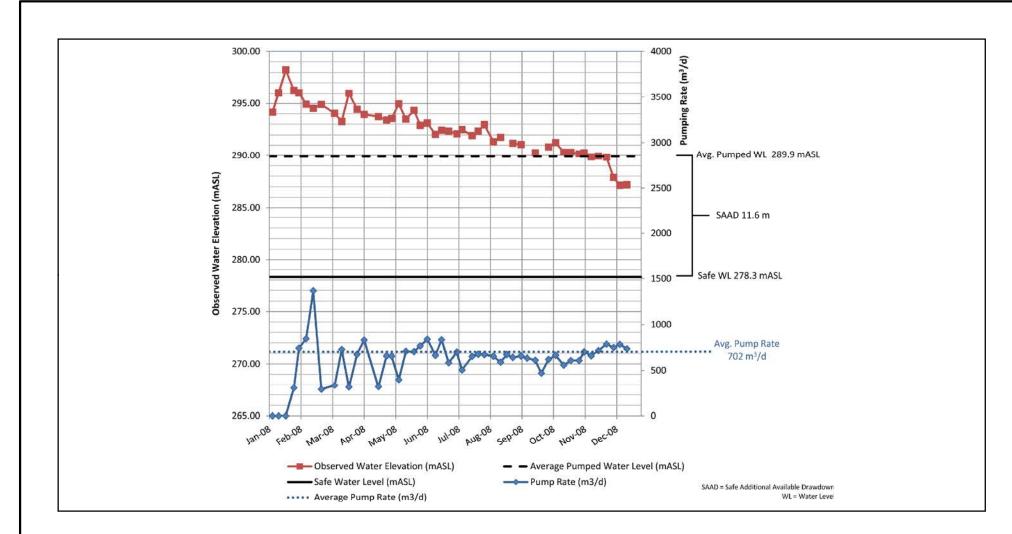
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Safe Additional Drawdown Calculation Park 1 and 2

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Safe Additional Drawdown Calculation Queensdale

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275.00

270.00

265.00

HOLOS MAYOS JUNOS

---- Observed Water Elevation (mASL)

Safe Water Level (mASL)

· · · · Average Pump Rate (m3/d)

2000

1000

- Average Pumped Water Level (mASL)

Pump Rate (m3/d)

Avg. Pump Rate 1,648 m3/d

SAAD = Safe Additional Available Drawdown



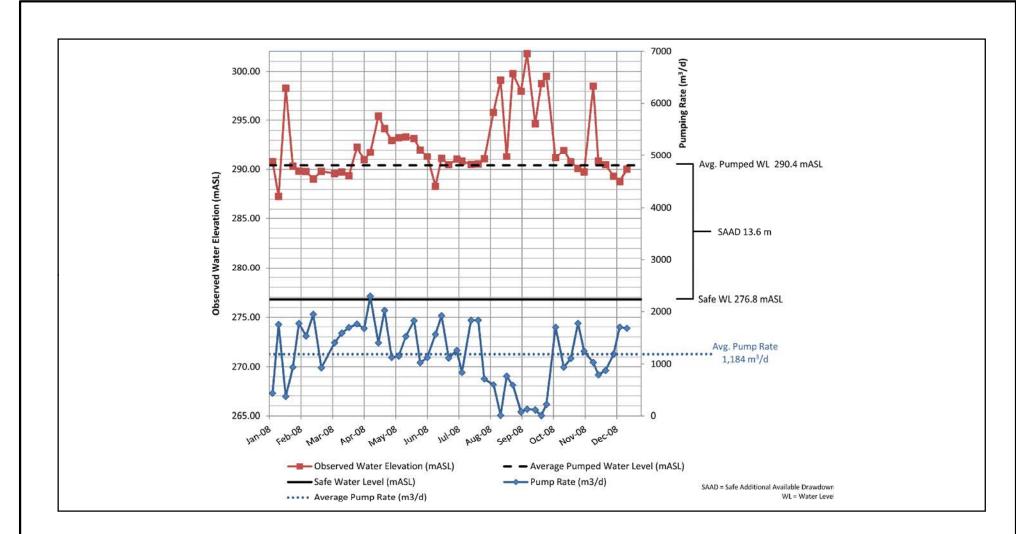
WL = Water Leve

City of Guelph and Township of Guelph/Eramosa Tier Three Water Budget and Local Area Risk Assessment Appendix F Safe Additional Available Drawdown

Safe Additional Drawdown Calculation University

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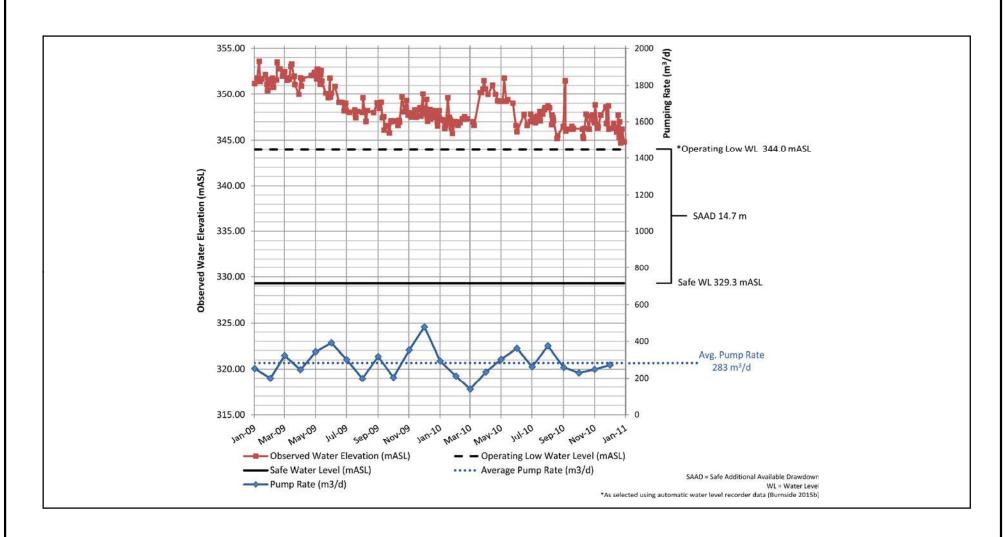
F17





Safe Additional Drawdown Calculation Water Street

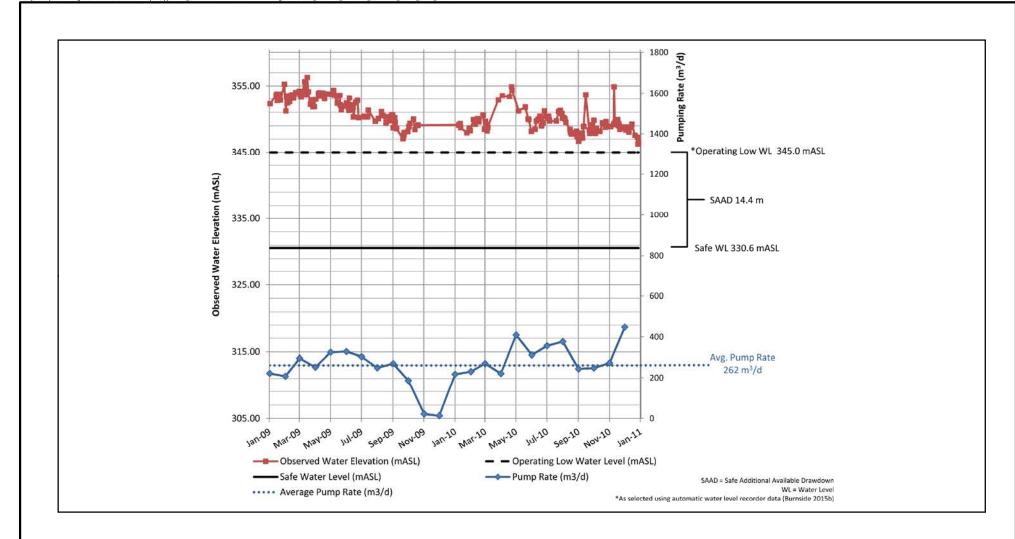
C. Curry 17 Jan 2017 J. Melchin Sclaimer: The information contained herein may be compiled from numerous third party materials that are subject to periodic change without prior notification. While every effort has been made by Matrix Solutions Inc. to ensure the accuracy of the information presented the time of publication, Matrix Solutions Inc. assumes no liability for any errors, omissions, or inaccuracies in the third party material.





Safe Additional Drawdown Calculation Rockwood 1

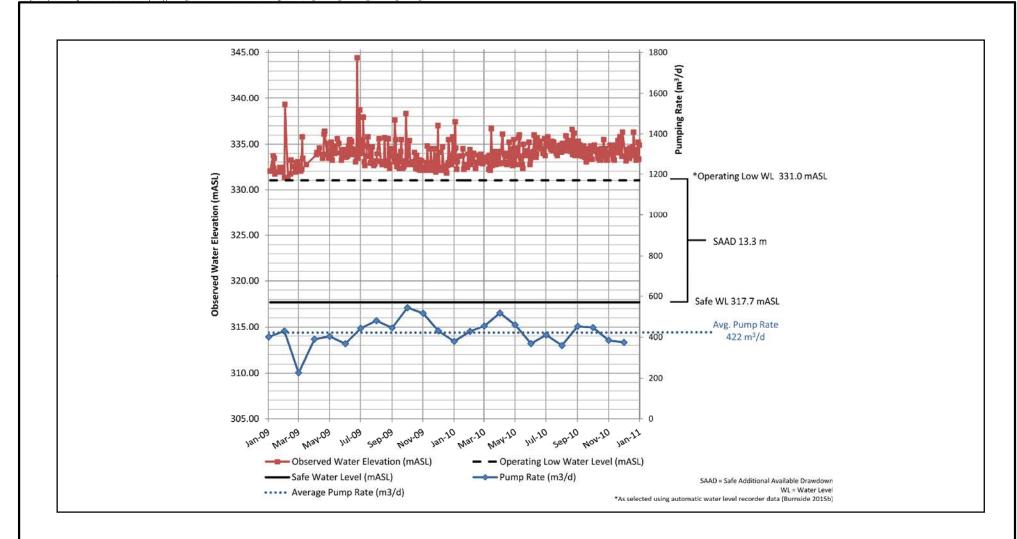
ate: 07 Mar 2017 Project 15072 Technical: J. Melchin Reviewer: P. Chin C. Curry schamer: The information contained herein may be compiled from numerous third party materials that are subject to periodic change Figure





Safe Additional Drawdown Calculation Rockwood 2

ate: 07 Mar 2017 Project: 15072 Technical: J. Melchin Reviewer: P. Chin Drawn: C. Curry

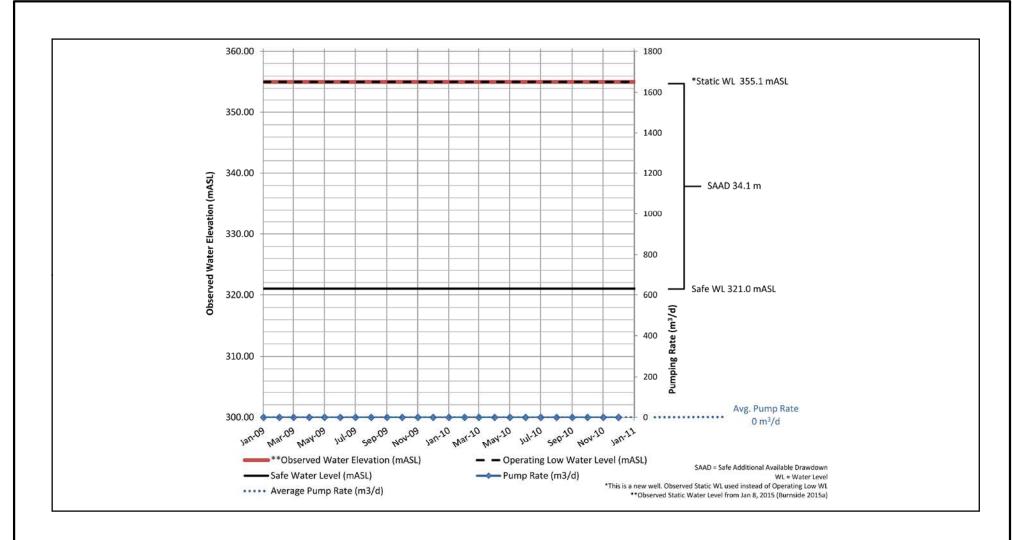




Safe Additional Drawdown Calculation Rockwood 3

ate: 07 Mar 2017 Project 15072 Technical: J. Melchin Reviewer: P. Chin C. Curry distance: The information contained herein may be compiled from numerous third pany materials that are subject to periodic change Figure

F21

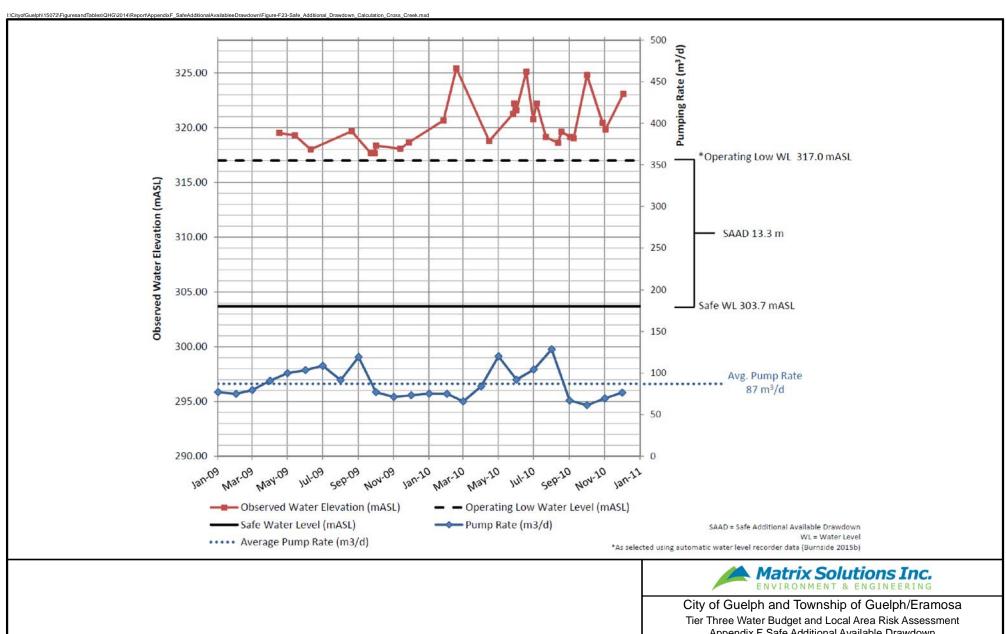




Safe Additional Drawdown Calculation Rockwood 4

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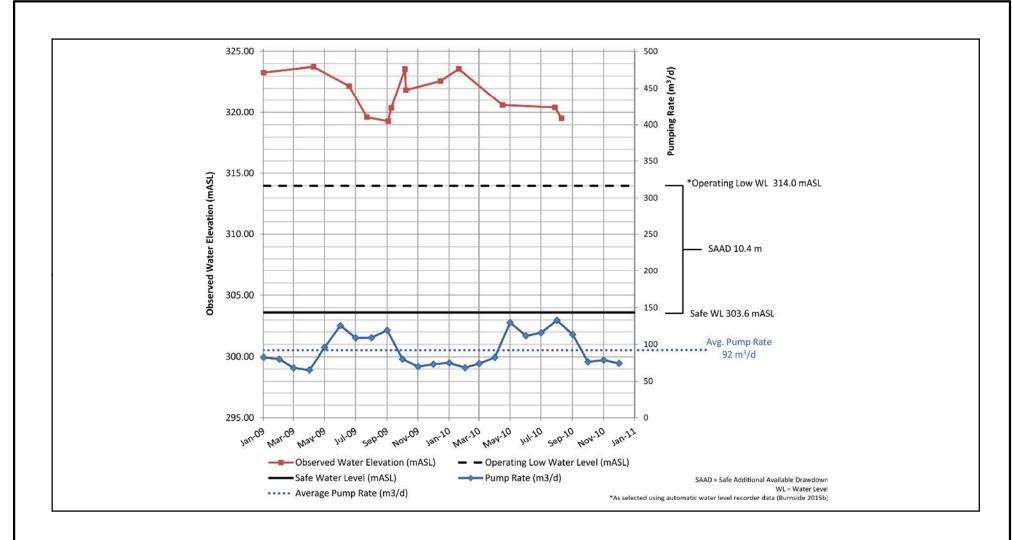
Appendix F Safe Additional Available Drawdown

Safe Additional Drawdown Calculation **Cross Creek**

ite:	17 Jan 2017	Project: 15072	Technical: J. Melchin	Reviewer: P. Chin	Drawn: M. Urtheil	
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F23

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Safe Additional Drawdown Calculation Huntington

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APPENDIX G Drought Scenario Results



City of Guelph and Township of Guelph/Eramosa Tier Three Water Budget and Local Area Risk Assessment Appendix G Drought Scenario Results

Drought Scenarios Predicted Drawdown - Arkell 1

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City of Guelph and Township of Guelph/Eramosa Tier Three Water Budget and Local Area Risk Assessment Appendix G Drought Scenario Results

Drought Scenarios Predicted Drawdown - Arkell 6

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Drought Scenarios Predicted Drawdown - Arkell 7

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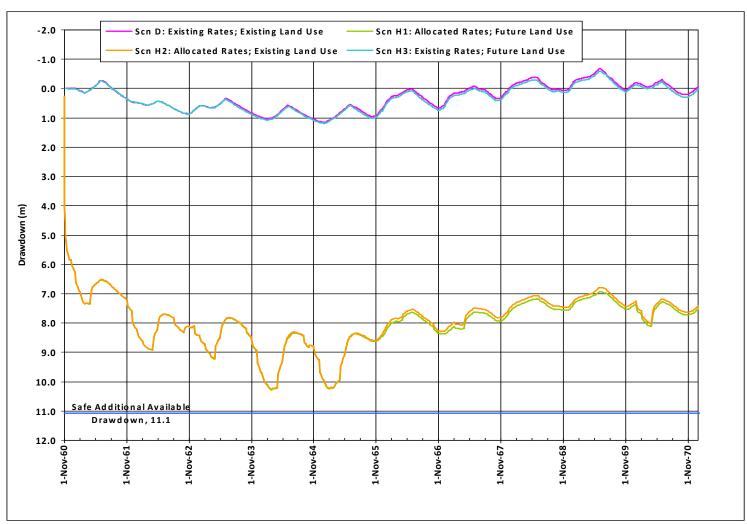
Drought Scenarios Predicted Drawdown - Arkell 8

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Drought Scenarios Predicted Drawdown - Arkell 14

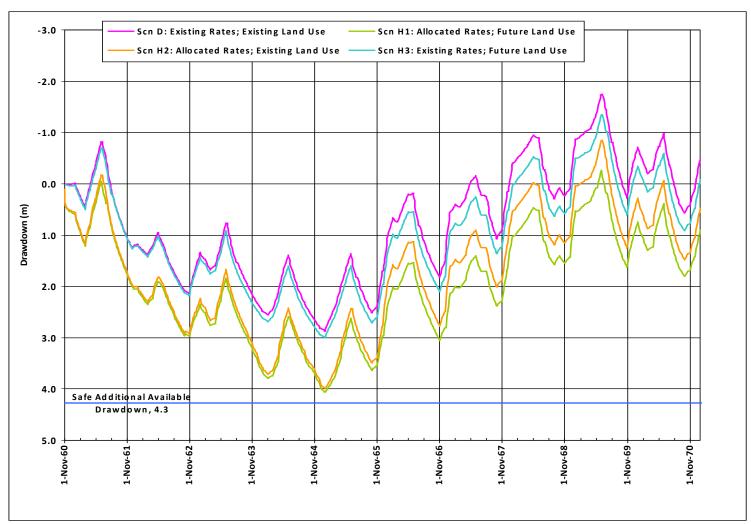
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Drought Scenarios Predicted Drawdown - Arkell 15

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Drought Scenarios Predicted Drawdown - Burke

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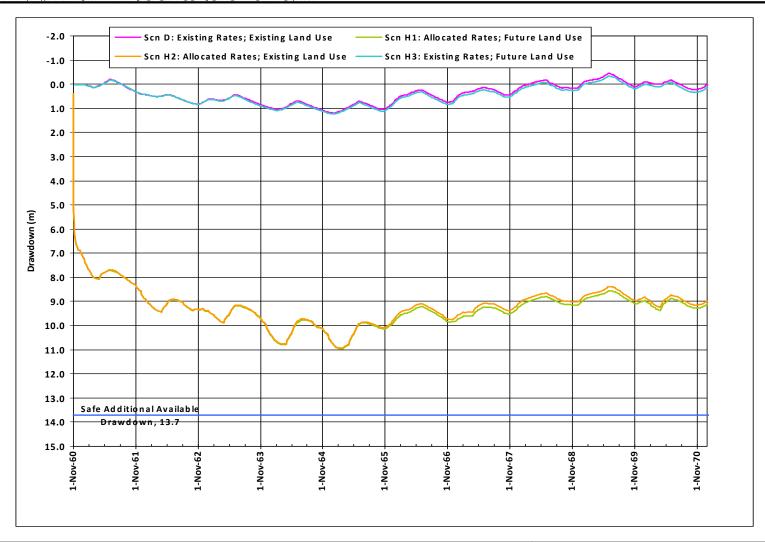
Drought Scenarios Predicted Drawdown - Calico

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without	prior notification. While eve	ery effort has been made by Matrix	n numerous third party materials the Solutions Inc. to ensure the accura iny errors, omissions, or inaccuracie	cy of the information presented at	



Drought Scenarios Predicted Drawdown - Carter Wells

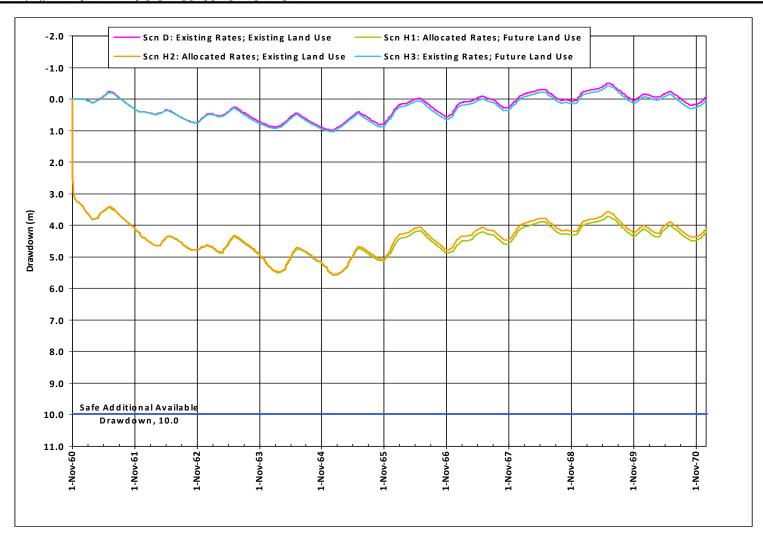
Date: 7 Mar 2017 Project 15072 Technical: J. Melchin Reviewer: P. Chin Dischaimer. The information contained herein may be compiled from numerous third party materials that are subject to periodic change willow up for rollification. While every effort has been made by Matrix Solutions in: to ensure the accuracy of the information presented at the time of publication, Marker Solutions in. assumers no leakily for any errors, comissions, or inaccuracions in the first party medical.





Drought Scenarios Predicted Drawdown - Clythe Creek

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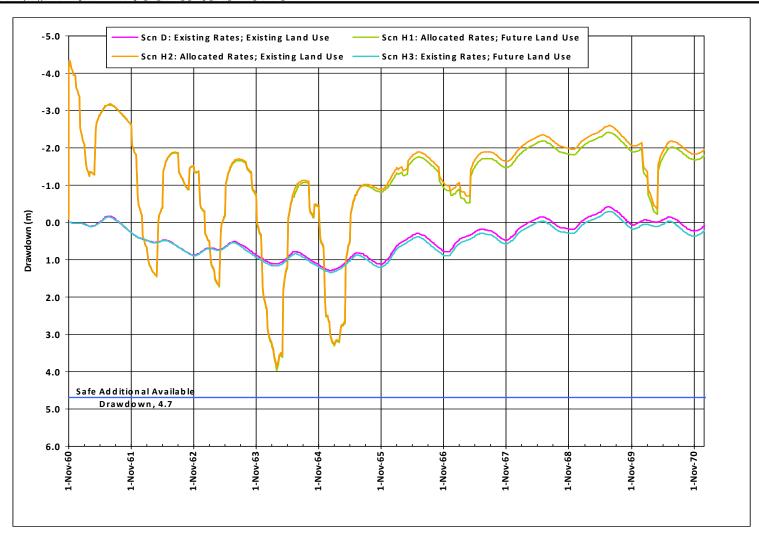
Drought Scenarios Predicted Drawdown - Dean Ave.

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Drought Scenarios Predicted Drawdown - Downey Rd.

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Drought Scenarios Predicted Drawdown - Emma

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Drought Scenarios Predicted Drawdown - Helmar

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Drought Scenarios Predicted Drawdown - Membro

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1-Nov-64

1-Nov-66

1-Nov-67

1-Nov-68

13.0 14.0

16.0 17.0 Safe Additional Available Drawdown, 15.2

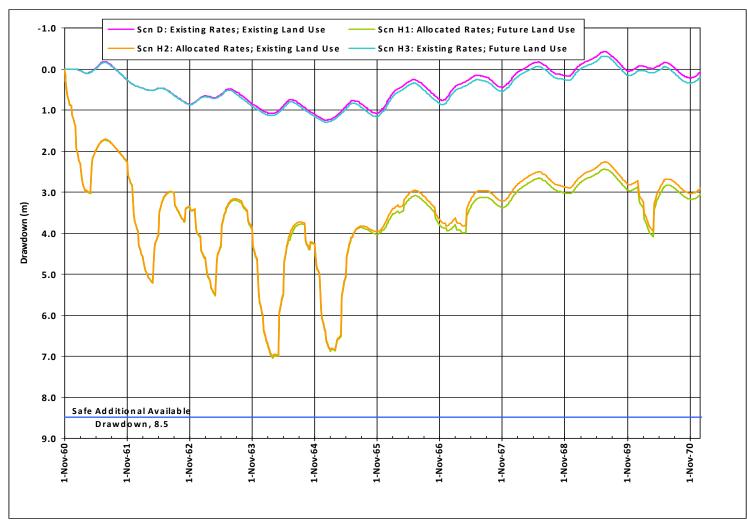
1-Nov-61



City of Guelph and Township of Guelph/Eramosa Tier Three Water Budget and Local Area Risk Assessment Appendix G Drought Scenario Results

Drought Scenarios Predicted Drawdown - Paisley

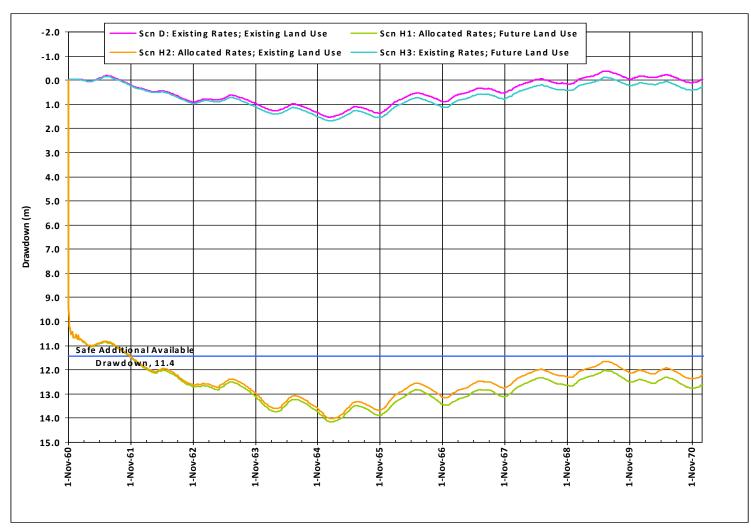
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Drought Scenarios Predicted Drawdown - Park 1 and 2

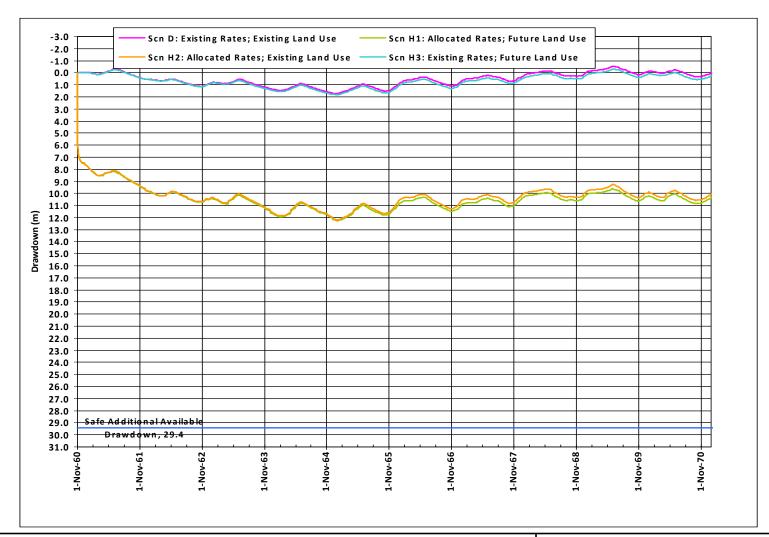
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Drought Scenarios Predicted Drawdown - Queensdale

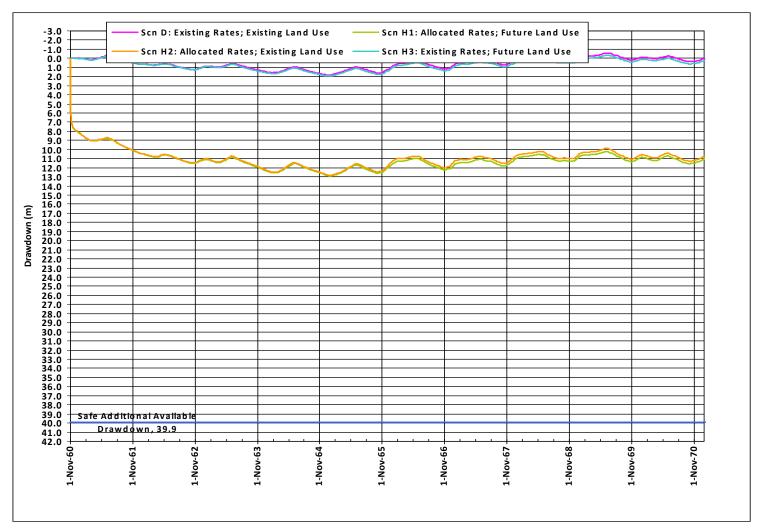
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Drought Scenarios Predicted Drawdown - Sacco

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Drought Scenarios Predicted Drawdown - Smallfield

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1-Nov-66

1-Nov-67

1-Nov-68

1-Nov-65

8.0 9.0 10.0 11.0 12.0 13.0

14.0 15.0 Safe Additional Available Drawdown, 13.4

1-Nov-61

1-Nov-63

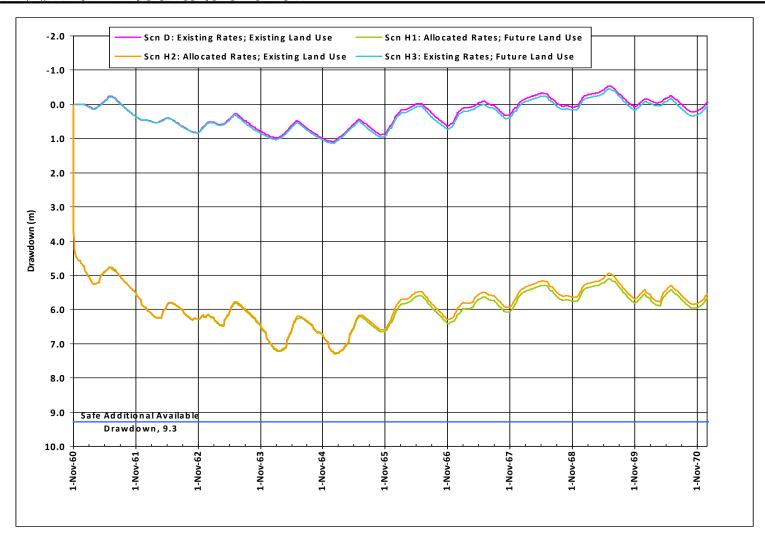
1-Nov-64



City of Guelph and Township of Guelph/Eramosa Tier Three Water Budget and Local Area Risk Assessment Appendix G Drought Scenario Results

Drought Scenarios Predicted Drawdown - University

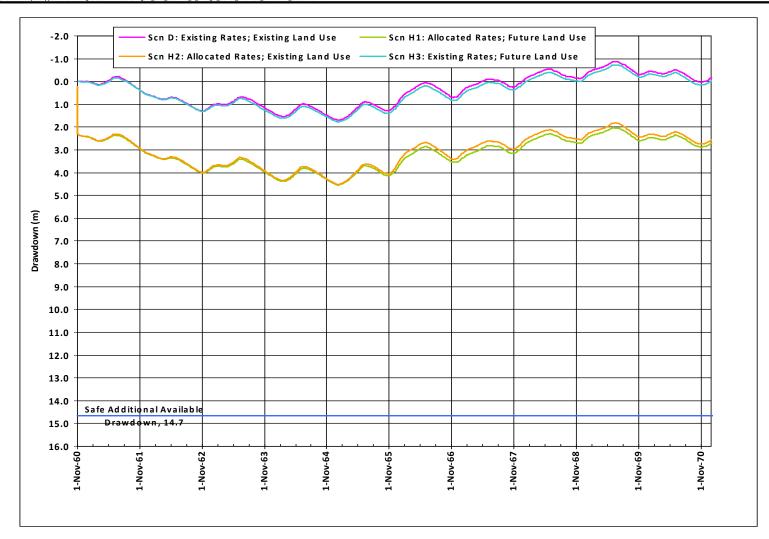
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Drought Scenarios Predicted Drawdown - Water St.

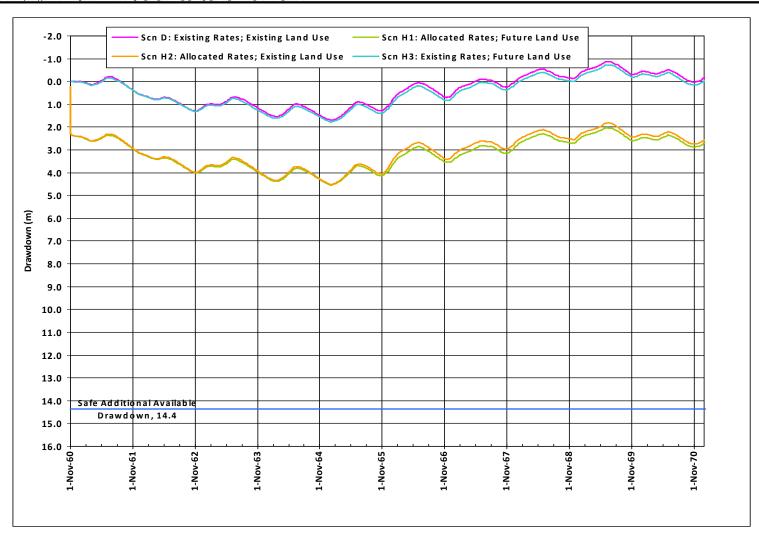
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Drought Scenarios Predicted Drawdown - Rockwood 1

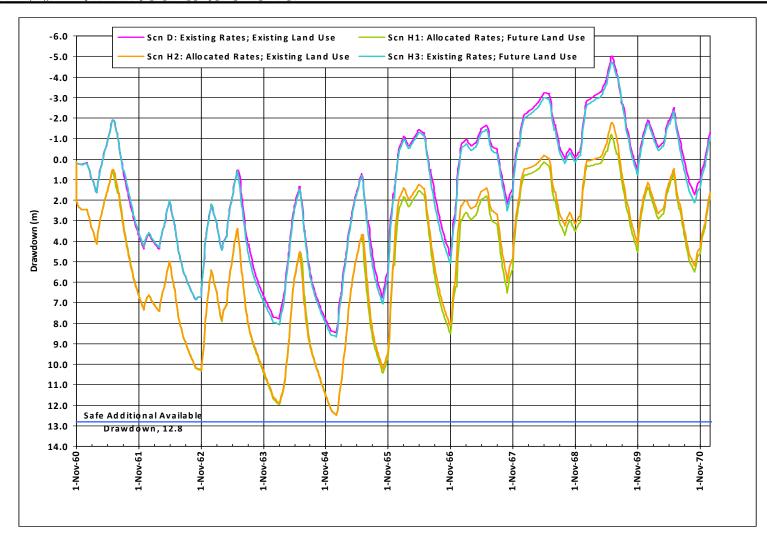
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Drought Scenarios Predicted Drawdown - Rockwood 2

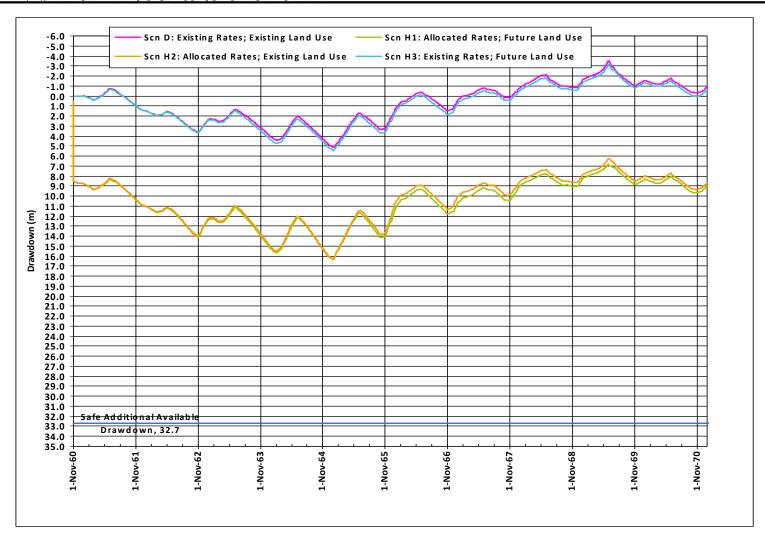
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Drought Scenarios Predicted Drawdown - Rockwood 3

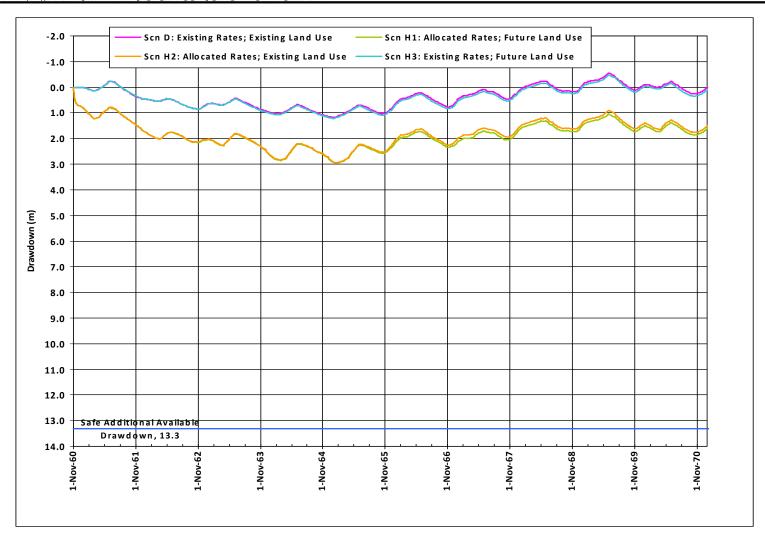
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Drought Scenarios Predicted Drawdown - Rockwood 4

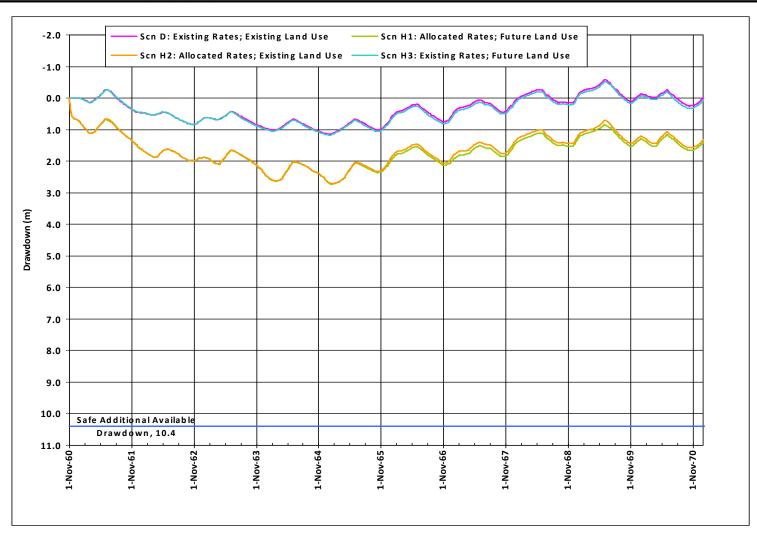
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Drought Scenarios Predicted Drawdown - Cross Creek

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Drought Scenarios Predicted Drawdown - Huntington

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